Berwick Acceleration Concept Report

31 March 2025



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1. INTRODUCTION

Northumbrian Water are required to reduce overflow spills at 31 overflows in the Berwick catchment to meet targets set out in the Storm Overflow Discharge Reduction Plan (SODRP) and PR24 Water Industry National Environment Programme (WINEP) drivers for storm overflows reductions. Northumbrian Water were awarded an Accelerated Infrastructure Delivery Project to make an early start on the scheme.

"The £1.85 million will fund model verification and feasibility work on proposed options and costs to determine how best to reduce overflow spills." Accelerated infrastructure delivery project, Final Decision, Ofwat, June 2023.

OFWAT set a number of requirements on the project:

- Provide evidence on where maintenance activities could reduce the overflow spills
- Disclose if any of the Berwick overflows require interventions to make them compliant with environmental permits
- Undertake further investigation to determine the root cause of spills
- Proportionally allocate costs between base and enhancement
- Modelling of the asset operation pre and post completion of the enhancement scheme and an explanation of the methodology and assumptions underpinning both sets of modelling.
- Consider opportunities for inclusion of nature-based and surface water management at source type solutions.
- Develop a best value solution to meet all investment drivers

Northumbrian Water submitted an interim report on compliance risk on 31 October 2024.

- There are no particular compliance issues with flow to full treatment, screen, storm tank or discharges from Berwick Sewage Treatment Works
- The 32 catchment combined sewer overflows meet their pass forward flow requirements when assessed on an individual basis assuming no downstream hydraulic restrictions.
- Investigation of the 16 Sewage Pumping Stations (SPSs) on the Berwick System was underway and had identified two mismatches at SPS 7 Magdalene Fields and SPS 9 Westfield between the design capacity of the pumps and the Flow Passed Forward (FPF) value in the permit.

This report summarises the findings of the Accelerated Infrastructure Delivery Project and includes options to resolve these stations from base funding.



2. DATA COLLECTION

Existing records, desk based studies and site surveys were collected as part of the study and are summarised in Table 1.

TABLE 1: SOURCES OF DATA

| Information | Source/Description | | |
|--|--|--|--|
| Mapping | From Northumbrian Water Elyx GIS | | |
| Network GIS | From Northumbrian Water Elyx GIS | | |
| Hydraulic Model | Verified model of the catchment used for the Drainage and Wastewater Management Plan. | | |
| Overflow Permits | Northumbrian Water Records | | |
| New developments | Provided by Northumbrian Water New Development and Local authority planning portal | | |
| Archaeology | A Heritage and Archaeological desk study was undertaken to identify constraints to any new works in the catchment | | |
| Environmental Constraints | Records of designated sites, habitats of principal importance and protected and notable species taken from Environmental Records Information Centre North East (ERIC North East). Habitat assessment conducted by AiDash - online maps processed using Arc | | |
| Geotechnical | A geotechnical desk study was undertaken for Berwick which identified geohazards including hazards relating to infiltration and poor ground conditions. | | |
| Utilities Information | Data obtained from National Underground Assets Register including Northumbrian Water sewers/water pipes, Northern Gas Networks, Northern Power Grid, Linesearch Before U Dig & others | | |
| NIDP study | Desk studies identifying historic instances of flooding, pollution and blockages within Berwick in addition to solutions optioneering to reduce instances of flooding etc. | | |
| A survey consisting of 95 flow monitors, 6 depth monitors and 8 rain gauges record rainfall, flow depth and velocity at locations within the network between 08/04/202Flow Survey15/07/2024 to analyse how the sewer network responds to storm events. The flow also recorded pipe diameters, depths to inverts to all pipes in the chamber and a cover of all the chambers where flow monitors were installed. | | | |
| Asset Surveys | Dimensional surveys off all pumping stations and combined sewer overflows were completed in April 2024. SPS drop tests were conducted in May 2024. Key manhole surveys, outfall surveys, were carried out April – May 2024. | | |
| Connectivity and Impermeable Area Surveys Connectivity surveys (identifying uncharted, private and highway drainage around proper and Impermeable Area Surveys (identifying the drainage paths of roof, road and other surfaces and their discharge point) improve understanding of the catchment and undertaken in May 2024 | | | |
| EDM Data | Event Duration Monitoring data was used for historic 12/24 spill counts and comparing model level and sewer level for verification storm events | | |
| Groundwater and mine water Project Groundwater Northumbria information | | | |



3. ROOT CAUSE ANALYSIS

3.1. APPROACH

:

The Storm Overflow Assessment Framework (SOAF) is a process for evaluating the impact of sewage spills and storm overflows on the environment. It is used to identify which overflows are most problematic and how to mitigate their impact. **THE STAGES IN THIS APPROACH ARE OUTLINED IN**



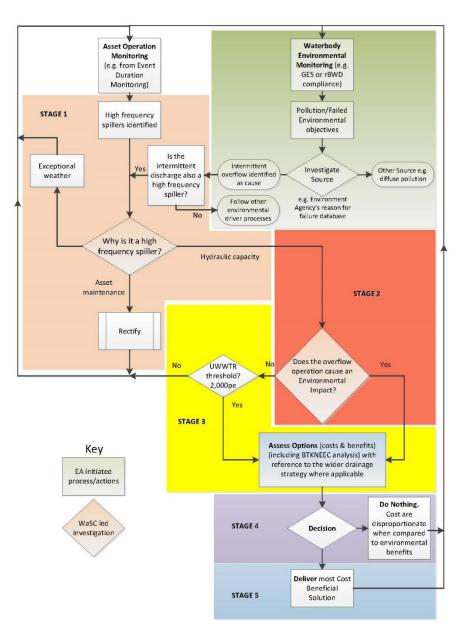


FIGURE 1: ASSESSMENT FRAMEWORK FOR ADDRESSING HIGH FREQUENCY DISCHARGES FROM STORM OVERFLOWS UNDER THE UWWTR, STORM OVERFLOW ASSESSMENT FRAMEWORK V1.6, ENVIRONMENT AGENCY, JUNE 2018

Stage 1 of the assessment examines "Why is it a high frequency spiller?" and identifies the factors that are causing frequent spills:

- exceptional weather
- asset maintenance
- hydraulic capacity

Exceptional weather



Catchment rainfall should be reviewed to determine whether rainfall was exceptional during any of the EDM reporting years. Any exceptional weather periods within the record were not extended enough to affect long term spill frequencies and is not the primary cause of high spill frequency.

Asset maintenance

This stage in the process is about investigating the asset and potentially parts of the upstream and downstream catchment to examine if the high spill frequency is the result of a maintenance issue.

TABLE 2 ASSET INSPECTION CONSIDERATIONS FOR STORM OVERFLOW MAINTENANCE ISSUES

| Investigation | |
|---|---|
| Storm overflows on gravity sewers | Review surveys for chamber and control condition Review CCTV for downstream sewer defects and blockages Records of blockages |
| Storm overflows at pumping stations | Pumping station pass forward complianceOperational issues |
| Storm overflows at sewage treatment works | Overflow arrangement Condition of inlet screens and actuated penstock Flow Pass Forward compliance |
| Infiltration assessments | Review mCerts for seasonal flow response due to groundwater or rainfall induced infiltration Review flow survey data for groundwater Operations knowledge |
| Crossed connections / misconnections investigation | Locations of dual manholes on GIS Unusual storm response in flow survey |

Hydraulic Capacity

A hydraulic model of the Berwick catchment was updated and verified for this project. This model was used to assess if hydraulic capacity is the primary cause of high spill frequency.



3.2. PERFORMANCE OF EXISTING OVERFLOWS

Table 3 lists the combined sewer overflows within Berwick and the primary cause of high spill frequency.

TABLE 3: BERWICK COMBINED SEWER OVERFLOWS

| CSO/PS No | STC Reference | Name | AMP | Туре | Permit | Primary cause of high spill frequency | Model Spill frequency - 2050 – |
|--------------|------------------|---|-----|----------------|--------------|--|--------------------------------------|
| CSO 1 | NU00516502 | SEA ROAD CSO | No | Hole-in-Wall | EPR/PP3227GJ | - | 2 |
| CSO 3 | NU00511806 | SPITTAL QUAY CSO (BT7) | 8 | High side weir | 210/1360 | Hydraulically Controlled | 44.1 |
| CSO 4 | NU00513805 | CAR PARK CSO | 8 | Hole-in-Wall | 210/1344 | Hydraulically Controlled | 11.8 |
| CSO 5 | NT99526102 | 33 DOCK ROAD CSO | 8 | Hole-in-Wall | 210/1345 | Hydraulically Controlled | 26.9 |
| CSO 6 | NT99519801 | BRANDYWELL CSO | 8 | Low side weir | 210/1359 | Hydraulically Controlled | 141.9 |
| CSO 7 | NU00515505 | MAIN STREET SPITTAL CSO | No | Hole-in-Wall | 210/1349 | - | 2 |
| CSO 8 | NT99523802 | CSO AT BLAKEWELL ROAD | 8 | Hole-in-Wall | 210/0938 | Hydraulically Controlled | 27.7 |
| CSO 9 | NT99525309 | MILL STRAND COMBINED SEWER OVERFLOW | 10 | Low side weir | 210/1348 | Hydraulically Controlled | 22.7 |
| CSO 10 | NT99524001 | MOUNT ROAD CSO BT003 | 8 | Low side weir | 210/1358 | Hydraulically Controlled | 28 |
| CSO 11 | NT99526101 | TOWER ROAD CSO | 8 | Hole-in-Wall | 210/0943 | - | 0.8 |
| CSO 12 | NT99528001 | DOCK ROAD CSO (BT44) | 8 | High side weir | 210/1363 | Hydraulically Controlled | 4.1 |
| CSO 13 | NT98525004 | TWEEDSIDE INDUSTRIAL ESTATE CSO | 8 | Low side weir | 210/1335 | Hydraulically Controlled | 77.4 |
| CSO 14 | NT99523700 | SSO AT WEST END | 8 | Hole-in-Wall | 210/0939 | Hydraulically Controlled | 24.8 |
| CSO 15 | NU00496501 | DERWENT WATER TERRACE CSO BT022 | 10 | Low side weir | EPR/AB3090VG | - | 0.4 |
| CSO 16 | NT99525301 | JNC CHURCH ROAD/DOCK ROAD CSO | 10 | Hole-in-Wall | 210/1346 | Hydraulically Controlled | 24.1 |
| CSO 17 | NT99529605 | PALACE STREET NORTH | 8 | Low side weir | 210/1361 | Hydraulically Controlled | 26.6 |
| CSO 18 | NT99545602 | MAGDALENE FIELDS NORTH CSO M2 monitoring | 10 | Low side weir | 210/1355 | Hydraulically Controlled | 115.3 |
| CSO 19 | NT99547300 | MAGDALEN FIELDS SOUTH COMBINED SEWW | 10 | High side weir | 210/1362 | Hydraulically Controlled | 130.8 |
| CSO 20 | NT99534501 | CASTLE TERRACE NO2 (BT341) CSO | No | Low side weir | WQD000697 | - | 2.6 |
| CSO 21 | NT99534502 | CASTLE TERRACE 1 CSO | 8 | Low side weir | 210/1356 | Hydraulically Controlled | 67.9 |
| CSO 22 | NT99529506 | PALACE STREET NORTH | 8 | Low side weir | 210/1357 | Hydraulically Controlled | 7.7 |
| CSO 23 | NU00523600 | BERWICK NO 4 SEWAGE PUMPING STATION | 8 | Hole-in-Wall | 210/1329 | - | 0 |
| CSO 24 | NT99545101 | WESTFIELD NO 9 PS | No | Low side weir | 210/0983 | - | 4.2 |
| CSO 26 | NT99535303 | RAILWAY STREET | 8 | Hole-in-Wall | 210/1350 | Hydraulically Controlled | 24.1 |
| CSO 27 | NT99524600 | QUEENS GARDENS CSO | 8 | Low side weir | 210/1007 | Hydraulically Controlled | 79.4 |



| CSO 28 | NT99524607 | BT41 MAIN STREET NO 4 CSO | 11 | Low side weir | 210/0940 | - | 0 |
|----------------|-------------------|--|----|--|------------------------------|-----------------------------|-------|
| CSO 29 | NT99528623 | SHOREGATE CSO | 8 | Transverse Weir | 210/1347 | - | 0.8 |
| CSO 31 | NT99528600 | QUAY WALL SPS SANDGATE CSO | 8 | High side weir | 210/0935 | Hydraulically Controlled | 99 |
| CSO 32 | NT99545604 | MAGDALENE FIELDS NORTH CSO High level M3 | 10 | Low side weir | Included in CSO 18 permit | Hydraulically Controlled | 64.9 |
| SPS 5 | NT99534505 | BERWICK SPS RAIL STATION NO 5 | No | Dry well centrifugal, overflow at inlet chamber | 210/1270 | Hydraulically Controlled | 115.5 |
| SPS 6 | NT98529504 | BERWICK SPS TWEEDSIDE TRADING ESTATE NO 6 | No | Dry well centrifugal, overflow at unknown point upstream | 210/1352 | - | 0 |
| CSO X | NU00541101 CSO | MAGDALENE FIELD | 10 | Low side weir | 210/1351 | Hydraulically Controlled | 27.5 |
| SPS 7 EO | NU00541104 | BERWICK (MAGDALENE FIELD) SPS NO 7 EO | 10 | submersible, overflow from wetwell | 210/1351 | - | 0 |
| SPS 8 | NU00532504 | BERWICK SPS CLUB HOUSE NO7A | No | 2 pump submersible, overflow from wetwell | 210/0960 | - | 0.1 |
| SPS 9 | NT99546103 | BERWICK SPS WESTFIELD NO 9 EO and SO | No | 2 pump submersible, overflow from wetwell | 210/0983 | Hydraulically Controlled | 18.9 |
| SPS 10 | NT97516503 | MILLFIELD SPS | 10 | | 210/A/0440 | Hydraulically Controlled | 95.2 |
| SPS 12 | NU00531004 | RAVENSDOWNE BARRACKS SPS | No | 2 pump submersible, overflow from wetwell | 210/1251 | - | 0 |
| SPS 13 | NT98504905 | TWEEDMOUTH SPS HIVEACRES | No | 2 pump submersible, overflow from wetwell | 210/1249 | - | 0 |
| SPS 14 | NT99534301 | CASTLE VALE SPS | No | 2 pump submersible, overflow from wetwell | 210/1000 | - | 0 |
| SPS 15 | NU00486803 | SCREMERSTON NO 1 SPS | 10 | | 210/0880 | - | 4.9 |
| SPS 16 | NU00499601 | SCREMERSTON NO 2 SPS | 10 | | EPRTB3996RM | Hydraulically Controlled | 46.6 |
| BERWICK STW | NT98522500 | BERWICK STW | 8 | Spill from storm tanks | 210/1273 | Hydraulically Controlled | 40.4 |

Model spill frequency is the average annual spill frequency calculated from the ten year series using the 2050 design horizon model.



4. BASELINE INVESTMENT

Northumbrian Water undertake regular inspection and maintenance activities designed according to the nature of the asset and risks and provide 24 hour cover to respond to e.g. telemetry alarms for reactive maintenance as required by permits. Northumbrian Water also operate an early warning system which monitors other levels in the system and flags up any potential issues that may result in a spill. An Asset Management Process System is in place to raise and track maintenance works.

In addition to regular inspections, NWG have a number of planned preventative maintenance initiatives in Berwick:

- Pledge 5 is a region wide project aimed at reducing spills from storm overflows to an average of 20 per year by 2025. Storm overflows are prioritised based on analysis of EDM spills and targeted inspection, maintenance and appropriate action is undertaken to mitigate spills.
- A bathing waters maintenance programme and consists of pre bathing season inspection and cleansing. This year the programme monitored 10 storm overflows in Berwick and CCTV surveyed and cleansed 3024m of network. A follow up survey is undertaken post bathing waters to monitor and assess if any further action is warranted.

These maintenance activities are aimed at reducing overflow spills and are funded through the base spend. Further maintenance identified as part of this study is discussed below.

Table 4 lists the works that have been undertaken at pumping stations to reduce compliance risk.

TABLE 4: WORKS UNDERTAKEN AT PUMPING STATIONS

| Pumping Station | Works undertaken | |
|----------------------|---|--|
| SPS 1 | Cleansing New impellors and wear plates. Volute and wear plate pump 2 | |
| SPS 2 | Cleansing 3 x new pump install. | |
| SPS 3 | Cleansing New impellors and wear plates. Wear parts and volutes on pump 1-3 | |
| SPS 7 | Cleansing New pumps New NRV | |
| SPS 11 | Cleansing New pumps | |
| Scremerston North | Cleansing New pumps Site refurbishment | |

Table 5 shows the operational and maintenance costs for interventions at the existing pumping stations.



TABLE 5: OPTIONS/COSTS FOR RESOLVING EXISTING PUMPING STATIONS

| Total | £113,000 |
|--------------------------|----------|
| Maintenance Intervention | £90,000 |
| Operational Intervention | £23,000 |

Investigation of the 16 SPSs on the Berwick System was underway and had identified two mismatches at SPS7 Magdalene Fields and SPS 9 Westfield between the design capacity of the pumps and the FPF value in the permit.

SPS 7 is permitted as a storm and emergency overflow and has a flow passed forward setting of 74 l/s. The storm overflow is located in a separate chamber immediately upstream of the station and an EO is present in the wet well. The SPS pumps to a combined sewer upstream of CSO 31 and SPS 3. The station currently passes 52 l/s. A Formula A value of 56.7 l/s would be achievable by changing pumps in the existing well and would not require upsizing downstream. SPS 7 is not designed for the permit of 74 l/s and would require a full station rebuild and new rising main. Furthermore, increasing the flow passed forward would result in detriment (flooding) in the receiving sewer. Works to increase the flow passed forward would require upsize of 551m combined sewer through Berwick old town, including crossing the Elizabethan Ramparts.

SPS 9 Westfield is permitted as a storm and emergency overflow and has a flow pass forward setting of 13.5 l/s. The station is located downstream of CSO 24 Westfield and pumps to SPS 7. The overflow is located in the wet well. The modelled 2050 spill frequency is 18 spills per year and is not identified for investment.

TABLE 6 OPTIONS TO RESOLVE SPS 7 AND SPS 9

| Asset | Description of Option | Advantages | Disadvantages |
|----------|--|--|--|
| SPS 7 | Revise permit to Formula A plus replace pumps | Pumps replaced with minimum disruption | Continues to spill higher then 10 spills per year until AMP 10 investment |
| | Station refurbishment (larger pumps) plus | Meets the permit | Passing more flow forward would increase spills to the estuary bathing waters at CSO 31 |
| | upsize downstream sewer | | Significant works required |
| | | | Works would be redundant once spill frequency solution is in place |
| SPS 9 | Revise permit to Formula A | No works required | The SPS is not identified for investment and spill frequency is higher than 10 spills |
| | Station refurbishment (pumps and main) | Meets the current permit Reduces spill frequency from the pumping station overflow | Passing more flow forward would increase spills at SPS 7 and to the estuary bathing waters at CSO 31 |

Northumbrian Water have commenced a dialogue with the EA on the optimum way of managing the compliance of these assets.



5. OPTIONS CONSIDERED

Table 7 shows the types of options considered for Berwick.

TABLE 7: OPTIONS CONSIDERED

| Option Considered | Description |
|--------------------------|--|
| Minor modifications | Including raising weirs, small amounts of pipe upsizing, opening up CSOs, |
| | upgrades to pumping stations |
| Source control | Measures near residential and commercial properties, highways, car parks |
| | and other paved areas including tree pits, rain gardens, bioretention and |
| | permeable paving |
| Disconnection of surface | Disconnecting SW sewers which connect to the combined system and |
| water systems | diverting to existing outfalls, with or without attenuation in detention basins, |
| | bioretention or geocellular storage |
| Separation of combined | Surface water removal by diverting surface water from combined sewers |
| sewer systems | to new separate surface water sewers connecting to existing outfalls, with |
| | or without attenuation in detention basins, bioretention or geocellular |
| | storage |
| Storage | Online/offline storage tanks |
| Storm overflow treatment | Treatment of storm overflows - only applicable at treatment works |

The approach to optioneering favours surface water removal through source control, disconnections and separation of combined sewer systems, using nature-based and surface water management at source type options where reasonably practicable. Green infrastructure measures follow the principles set out in the SuDS manual. Options have been presented to local planning and highways authorities to gain feedback.

The factors affecting provision of green infrastructure are:

- Practicalities of accessing the surface water source, space to build including tree canopy, ability to infiltrate or return of flows to a network or watercourse
- Cost and likely benefits the measures would return
- Environmental and archaeological constraints and impact on the community e.g. loss of car parking space
- Estimate of the impermeable area that measures can intercept and potential impact on spill frequency.



The shortlisting criteria from the Surface Water Management Plan Technical Guidance (Figure 2) was used to identify opportunities with highest potential.

| Criteria | Description | Score | | |
|-----------------------------|--|---|--|--|
| Technical | Is it technically possible and buildable? Will it be robust and reliable? | U (unacceptable) – measure eliminated from further consideration | | |
| Economic | Will benefits exceed costs? | - 2 severe negative outcome | | |
| Social | Will the community benefit or suffer from implementation of the measure? | 1 moderate negative outcome +1 moderate positive outcome | | |
| Environmental ⁴³ | Will the environment benefit or suffer from implementation of the measure? | +2 high positive outcome | | |
| Objectives | Will it help to achieve the objectives of the SWMP partnership? | | | |

FIGURE 2 EXAMPLE OF SHORT-LISTING CRITERIA FROM SURFACE WATER MANAGEMENT PLAN TECHNICAL GUIDANCE, DEFRA, MARCH 2010

Any excess spills following surface water removal were addressed by storage. The approach to developing storage options was

- Group storm overflows to one storage area
- Find space to build
- Determine if online storage is feasible or offline storage with pump return is required
- Consideration of capacity of the network and STW to drain down tanks

The project produced concept stage schematic designs for the purposes of pricing and evaluation, to be developed at the next project stage.

Options were developed with the benefit of a verified hydraulic model. Model assumptions are summarised in Table 8.



TABLE 8 HYDRAULIC MODELLING ASSUMPTIONS

| Modelling Input | Base (2025) | 5 Year Planning Horizon (2030) | 25 Year Planning Horizon (2050) |
|-------------------------------|---|--|---|
| Growth | Short-term committed development to be provided by NWG. | Local Plan / SHLAA data to be provided b | y NWG and used to update hydraulic model. |
| Urban Creep | No application of urban creep. | To be updated in accordance with the UK | WIR methodology. |
| Design Rainfall | FEH22 The following return period storms DWF 1 in 20 year and 1 in 40 year | will be simulated and used for level of serv | vice checks and flood risk assessment. |
| Time Series Rainfall | Ten-year TSR dataset for Berwick has bee rain gauge at Berwick STW | en produced for the current day from the | 2050 TSR dataset has been generated which has been updated for the impacts of climate change using the UK Water Industry Research (UKWIR) climate perturbation tool, 'RedUP'. |
| Climate Change Uplift | Not applicable. | Not applicable. | 25% uplift to be applied to all (summer and winter) design storms, in line with Environment Agency central allowance for Tweed Management catchment. |
| Infiltration | Modelled base flows to be calibrated against the short-term flow survey. This will also be checked against MCERTS data. | As Base Model. | |
| Per Capita Consumption | 138l/hd/day, default CIRIA profile to be app | plied in all of the planning horizon hydraulio | c model simulations. |
| Trade / Industrial Flows | Latest available information (Feb 2024) provided by NWG. | As Base Model. | |
| Boundary Conditions / Tide | Applied, where appropriate. Propose to use mean high tide for TSR Simulations Mean High Water Springs for design events. Applied, where appropriate, during design storms only. | Uplifted in line with allowance for Northumbria catchment <u>Flood risk</u> <u>assessments: climate change</u> <u>allowances - GOV.UK</u> | Uplifted in line with allowance for Northumbria catchment <u>Flood risk</u> <u>assessments: climate change allowances</u> <u>- GOV.UK</u> |

6. EVALUATION

6.1. OPTIMISATION

The best value solution is the combination of options which resolves the storm overflows at the lowest cost:benefit i.e. taking account of the wider benefits to the community and environment. Modefrontier design optimisation software and Infoworks ICM were used to test the combination of options and evaluate costs and benefits of those optionst. The optimiser identifies a number of combinations that meet the spill frequency criteria at the lowest cost. These solutions may then be assessed in more detail.

The variables that were assessed during optimisation are listed in Table 9:

TABLE 9: VARIABLES ASSESSED DURING OPTIMISATION

| CSO weir raised | up to a level maintaining existing levels of service |
|--|---|
| Conduit upsize downstream of CSOs | to allow passing flow forward |
| Modification of pass forward flows at CSOs | to allow limiting or increasing flows, while maintaining formula A |
| Pump upgrades at all stations | assumes 10% uplift is achievable with current installation and above this requires a new station |
| Surface Water Removal across the catchment | based on reasonably practicable |
| Storage at 12 locations and the works | space is available at 4, 6, 10, 12, 16, 18, 19, 21, 24, 27, 31, SPS16. |

The preferred option from the best value solutions identified by the optimiser minimises storage and maximises the wider benefits value from implementing surface water removal solutions.



6.2. STORM OVERFLOW PERFORMANCE

Table 10 shows the predicted spill frequency post completion of the enhancement scheme against a 2050 design horizon.

TABLE 10: SPILL FREQUENCY AT OVERFLOWS FOLLOWING INTERVENTIONS

| CSO/PS No | STC Reference | Name | AMP | Bathing Water/ Inland | Annual/BW Spill frequency 2050 | Storage required? | Screen required? | Summary | |
|-----------|------------------|---|-------------|-----------------------------|---|-------------------|--------------------------------------|---|--|
| CSO 1 | NU00516502 | SEA ROAD BT 008 | No AMP | BW | 0 | | | Overflow may be abandoned | |
| CSO 3 | NU00511806 | DOCK ROAD SPITTAL QUAY) BT007 | AMP 8 | BW | 2 | Yes | Static screen | New chamber and storage | |
| CSO 4 | NU00513805 | SANDSTELL ROAD | AMP 8 | BW | 0 | | Static screen | Overflow may be abandoned | |
| CSO 5 | NT99526102 | BERWICK DOCK ROAD NO 33 | AMP 8 | BW | 0 | | Static screen | Retained as high level overflow (screen TBD) | |
| CSO 6 | NT99519801 | SPITTAL BT 005 | AMP 8 BW | BW | 2 | Yes | Mechanical screen | New chamber and storage | |
| CSO 7 | NU00515505 | MAIN STREET BLENHEIM PLACE | AMP | BW | 0 | | | Retained as high level overflow. Do nothing (no AMP). | |
| CSO 8 | NT99523802 | BLAKEWELL ROAD BT 042 | AMP 8 | Inland | 0 | | Static screen | Overflow may be abandoned | |
| CSO 9 | NT99525309 | MILL STRAND (NO3) | AMP 10 | BW | 0 | | | Overflow may be abandoned | |
| CSO 10 | NT99524001 | MOUNT ROAD CSO BT003 | AMP 8 | BW | 1.5 | | Static screen | Retained as high level overflow (screen TBD) | |
| CSO 11 | NT99526101 | CSO - BT 002 Tower Rd, Tweedmouth. | AMP 8 | BW | 0 | | Static screen | Overflow may be abandoned | |
| CSO 12 | NT99528001 | DOCK ROAD NO 39 BT 044 | AMP 8 | BW | 2 | Yes | Static screen | New chamber and storage | |
| CSO 13 | NT98525004 | TWEEDSIDE INDUSTRIAL ESTATE BT 004 | AMP 8 | Inland | 10 | Yes | Static screen | Retrofit | |
| CSO 14 | NT99523700 | WEST END (NO67) BT 043 | AMP 8 | Inland | 0 | | Static Overflow may screen abandoned | | |
| CSO 15 | NU00496501 | DERWENT WATER TCE SCREMERSTON BT 022 | AMP 10 | Inland | 0 | | | Do nothing (AMP 10) | |
| CSO 16 | NT99525301 | CHURCH ROAD DOCK ROAD | AMP 10 | BW | 10 | Yes | | New chamber and storage | |
| CSO 17 | NT99529605 | PALACE STREET EAST (NO4) BT038 | AMP 8 | BW | 0 | | Static screen | Overflow may be abandoned | |
| CSO 18 | NT99545602 | MAGDALEN FIELDS A (NORTH) BT 029 | AMP 10 | Inland | 10 | Yes | | Do nothing (AMP 10) | |
| CSO 19 | NT99547300 | | | Inland | 10 | Yes | | Do nothing (AMP 10) | |
| CSO 20 | NT99534501 | CASTLE TERRACE (EAST) BT 341 | | Inland | 0 | | | Overflow may be abandoned | |
| CSO 21 | NT99534502 | CASTLE TERRACE (EAST) BT 034 | | Inland | 10 | Yes | Static screen | New chamber and storage | |
| CSO 22 | NT99529506 | PALACE STREET EAST PALACE STREET BT381 | AMP 8 | BW | 0 | | Static screen | Overflow may be abandoned | |
| CSO 23 | NU00523600 | | AMP 8 | BW | 0 | | Static screen | New chamber | |



| CSO 24 | | WESTFIELD ROAD (NO6) BT 032 | No AMP | Inland | 2.4 | Yes | | Do nothing (no AMP) | |
|---------------------------------|------------|---|-----------|--------|-----|-----|-------------------|---|--|
| CSO 26 | NT99535303 | RAILWAY STREET (NO16) TWEED STREET BT 031 | AMP 8 | Inland | 3 | | Static screen | New chamber | |
| CSO 27 | NT99524600 | QUEENS GARDENS TWEEDMOUTH BT 040 | AMP 8 | Inland | 10 | Yes | Static screen | New chamber and storage | |
| CSO 28 | NT99524607 | WEST END ROAD, TWEEDMOUTH BT 041 | AMP 11 | Inland | 0 | | | Overflow may be abandoned | |
| CSO 29 | NT99528623 | SHORE GATE QUAYSIDE | AMP 8 | BW | 0 | | Static screen | Overflow may be abandoned | |
| CSO 31 | NT99528600 | SANDGATE, BERWICK BT 033 | AMP 8 | BW | 2 | Yes | Mechanical screen | New chamber and storage | |
| CSO 32 | | MAGDALENE FIELDS NORTH, BERWICK | AMP 10 | Inland | 0 | | | Do Nothing (AMP 10) | |
| SPS 5 EO | NT99534505 | BERWICK SPS RAIL STATION NO 5 | No AMP | Inland | 0 | | | Requires upgrade to facilitate CSO20/21 works | |
| SPS 6 EO | | BERWICK SPS TWEEDSIDE TRADING ESTATE | No AMP | Inland | 0 | | | Overflow not found | |
| CSO X | CSO | BERWICK CSO MAGDALENE FIELD | AMP 10 | Inland | 8.7 | Yes | | Do Nothing (AMP 10) | |
| SPS 7 EO | | BERWICK (MAGDALENE FIELD) SPS NO 7 | AMP 10 | Inland | 0 | | | May require works to meet permit | |
| SPS 8 EO | | BERWICK SPS CLUB HOUSE NO7A | No AMP | Inland | 0.1 | | | Do nothing (no AMP) | |
| and SO | NT99546103 | BERWICK SPS WESTFIELD NO 9 | No AMP | Inland | 0 | | | May require works to meet permit | |
| and SO | NT97516503 | BERWICK SPS EAST ORD NO 10 | AMP 10 | Inland | 10 | Yes | | Do Nothing (AMP 10) | |
| and SO | NU00531004 | BERWICK SPS RAVENSDOWNE BARRACKS | No AMP | BW | 0 | | | Do nothing (no AMP) | |
| SPS 13 EO | NT98504905 | TWEEDMOUTH SPS HIVEACRES | No AMP | BW | 0 | | | Do nothing (no AMP) | |
| SPS 14 EO and SO | NT99534301 | BERWICK SPS CASTLE VALE RESIDENTIAL HOME | No AMP | Inland | 0 | | | Do nothing (no AMP) | |
| and SO | | SCREMERSTON SOUTH SPS 1 | AMP 10 | Inland | 3.1 | | | Do nothing (AMP 10) | |
| and SO | | SCREMERSTON NORTH SPS 2 | AMP 10 | Inland | 10 | Yes | | Do nothing (AMP 10) | |
| BERWICK UPON TWEED STW | NT98522500 | BERWICK UPON TWEED STW | AMP 8 | BW | 2 | Yes | | Additional storm tanks | |

The following pumping stations have no physical overflow and share an overflow with the upstream CSO:

SPS1, SPS2, SPS 3, SPS 4, SPS 11

Annual/BW Spill frequency 2050 is the average annual spill frequency for inland discharges or average bathing season spill frequency for bathing water discharges calculated from the ten year series using the 2050 solution model and using the 12/24 spill count method.



6.3. COSTS AND BENEFITS

A summary of the costs and monetised benefits for the project are shown in Error! Reference source not found.

TABLE 11 SUMMARY OF COSTS AND BENEFITS FOR THE WHOLE CATCHMENT SOLUTION

| Option | Brief Description of Option | CAPEX | OPEX | Benefits | NPV | WHOLE LIFE CARBON (tCO2e) |
|--|---|-----------------------------|--------------|--|-------|------------------------------------|
| Minor modifications Surface Water Removal | Raising weirs/abandonment of 15 storm overflowsRemoval of 398,681m² of impermeable area across catchment | £115m whole catchment | £0.8m NPV | £45m Flood risk benefit to 758 residential and commercial properties, | -£47m | 15,000 |
| Pumping Station Upgrade Storage | Upgrade at SPS 5 to manage storage tank draindown 7,899m ³ storage at 12 locations within the network 3,840m ³ storage at works | | | biodiversity net gain | | |

CAPEX is the total project cost for delivering the scheme as a present value and includes for estimating uncertainty and risk. Costs are based on unit rates for infrastructure items in the NWG HIVE Market Rate application applied to bill of quantities values. Where cost data was unavailable, Unit Rate Costs from various sources across the UK Water Industry recording Base Date and BCIS Inflation factors applied to 2025 Q1 were used.

OPEX is the projected operational cost for maintaining the works over a 40 year period, discounted to a present day value.

Benefits value is the quantified wider benefits associated predominantly with green infrastructure for biodiversity improvement, carbon, amenity improvements, education and flood risk reduction. Monetised benefits for amenity and education have been calculated using CIRIA's "benefits estimation tool - B£ST" (CIRIA 2024). Biodiversity benefits have been calculated from "Enabling a Natural Capital Approach (ENCA) 2024 Services Databook" (DEFRA 2025) and carbon benefits from "The Department for Energy Security and Net Zero's Traded carbon values used for modelling purposes" (DESNZ 2023). Flood risk benefits were measured in terms of changes to Annual Average Damages (AAD).



The results of the flood risk assessment suggest that there are around 658 residential and 100 non-residential properties with a significantly reduced risk of flooding as a result of the catchment solution. These are generally properties that are currently at risk of internal or external flooding in a 1 in 20 year event that will move to no risk in the 2050 solution model.

Whole Life Carbon is the total capital embodied carbon and the operational carbon over the life of the asset, taken as 40 years.

Embodied carbon values were assessed using "A Framework for Accounting for Embodied Carbon in Water Industry Assets" (UKWIR 2012, 2022) and the HM Treasury's Green Book (HM Treasury, 2022) and its supplementary guidance (DESNZ, 2023) for undertaking a carbon assessment. Carbon emissions are measured in kgCO2e and values are taken from environmental product declarations (EPDs) produced by suppliers. Where EPDs were not available, values were assumed based on carbon values for similar materials/components.

7. CONCLUSIONS

- Northumbrian Water have in place a robust monitoring and planned preventative maintenance and specific initiatives to reduce the risk of spills at Berwick.
- This project identified further investment at key pumping stations to reduce the risk of non-compliance. These maintenance activities are funded from base.
- Investigation of the 16 SPSs on the Berwick System identified two mismatches at SPS7 Magdalene Fields and SPS
 9 Westfield between the design capacity of the pumps and the FPF value in the permit. Northumbrian Water have opened up discussions with the Environment Agency to determine the best course of action. This would be delivered from base funding.
- Data collection including a survey of all pumping stations and storm overflows, targeted manhole surveys and CCTV and a catchment wide flow survey were undertaken to inform the investigation.
- A hydraulic model of the Berwick catchment was updated with additional data collection and verified against the new flow survey for this study. The model was used to assess asset operation pre and post completion of the enhancement scheme against a 2050 design horizon.
- A root cause assessment of spills identifies hydraulic control as the primary source of high spill frequency at the driver overflows
- The approach to optioneering favours surface water removal through source control, disconnections and separation of combined sewer systems, using nature-based and surface water management at source type options where reasonably practicable
- An optimiser was used to assess the combinations of options across the catchment and identify the best value solutions that deliver the drivers

